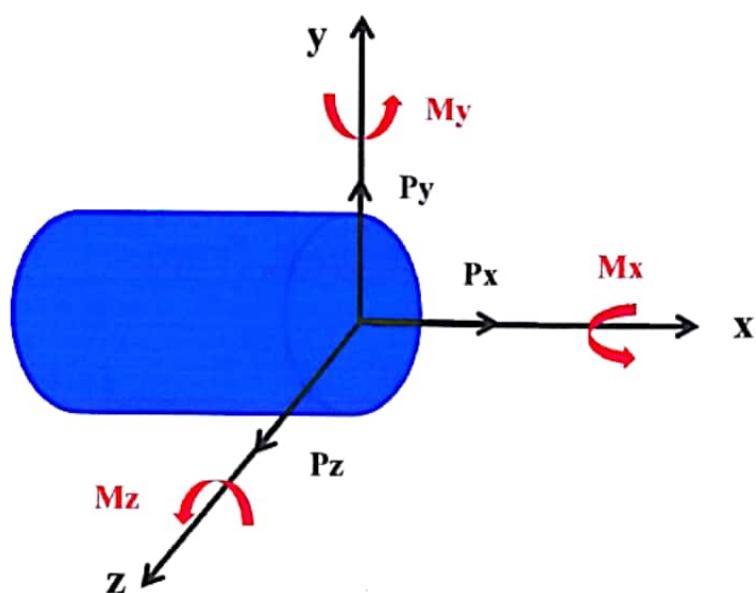


Internal Forces

For any equilibrium body, if we take any section perpendicular to its longitudinal axis, a system of six internal forces will be obtained, as shown in fig. These forces can be classified into four types:



1- Axial force or normal force (P_x): represents a tensile or compressive force in (X) direction (acts perpendicular to that cross-section) and usually denoted by (N).

2- Shear force (P_y and P_z): acts in plane of that cross-section (i.e. tangential), usually denoted by (V), and tends to slide the two parts of the body.

$$VR = \sqrt{P^2y + P^2z}$$

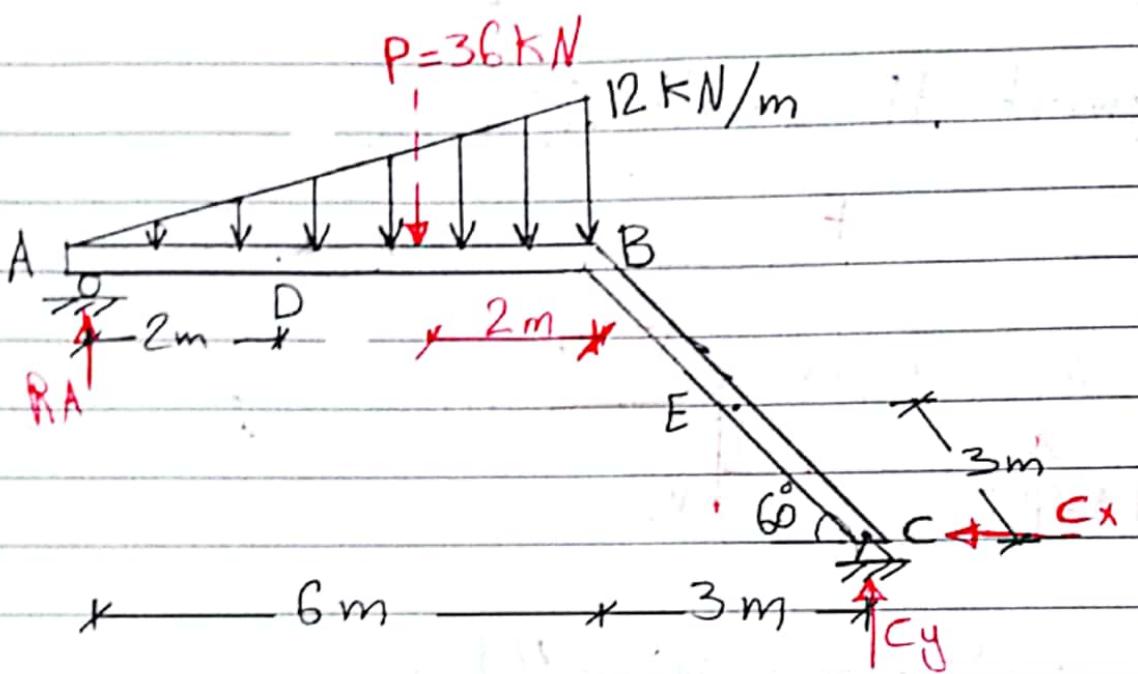
3-Torsional moment (or torque) M_x : tends to twist one segment of the body with respect to other, and usually denoted by (M_t or T).

4- Bending moment (M_y, M_z): that tends to bend the body about (Y) or (Z) axis.

$$MR = \sqrt{M^2y + M^2z}$$

Ex1: For the structure shown in Fig., determine the internal forces and their types acting on the cross-section of (D) and (E).

Sol



* Find external reactions:

$$\sum F_x = 0 \rightarrow C_x = 0$$

The whole structure as F.B.D.:

$$\textcircled{+} \sum M_c = 0$$

$$RA \cdot 9 - P \cdot 5 = 0$$

$$RA \cdot 9 - (\frac{1}{2} \cdot 6 \cdot 12) \cdot 5 = 0 \rightarrow RA = 20 \text{ kN} \uparrow$$

$$+\uparrow \sum F_y = 0$$

$$RA + Cy - P = 0$$

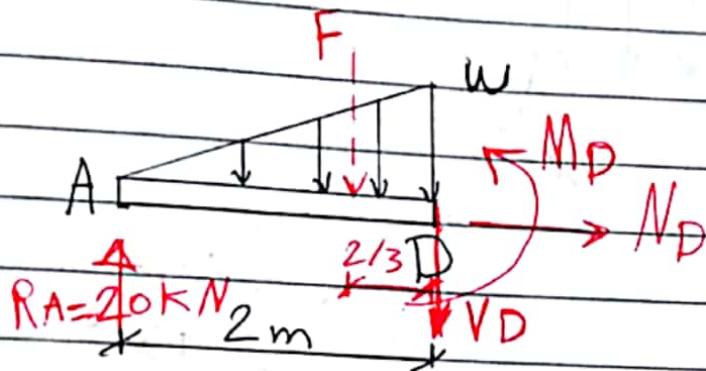
$$20 + Cy - 36 = 0 \rightarrow$$

$$Cy = 16 \text{ kN} \uparrow$$

* Take the considered section and put the internal forces:

I - cross-section of (D):

Segment AD as f.B.D



$$\frac{12}{6} = \frac{w}{2} \rightarrow w = 4 \text{ kN}$$

$$F = \frac{1}{2} \times 2 \times 4 = 4 \text{ kN}$$

$$\sum F_x = 0 \rightarrow N_D = 0 \quad (\text{no axial force})$$

$$+ \uparrow \sum F_y = 0 \rightarrow R_A - V_D - F = 0$$

$$20 - V_D - 4 = 0 \rightarrow V_D = 16 \text{ kN} \quad (\text{shear force})$$

$$+ \leftarrow \sum M_D = 0$$

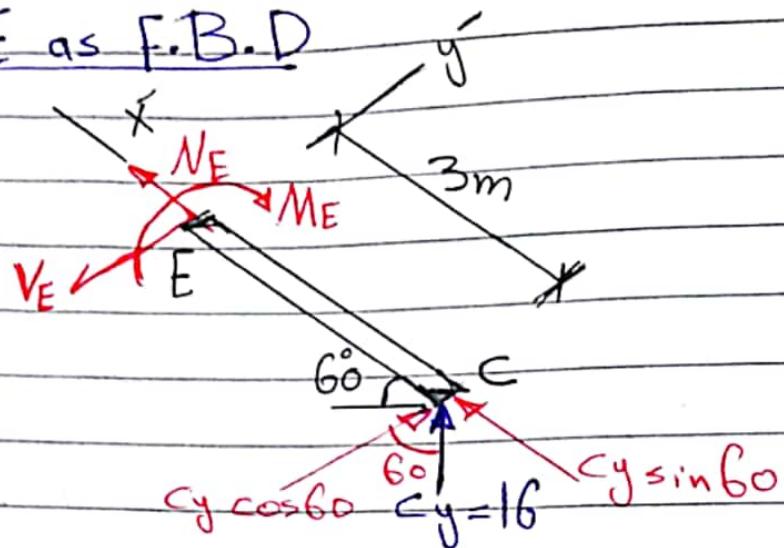
$$M_D - R_A \cdot 2 + F \cdot \frac{2}{3} = 0$$

$$M_D - 20 \cdot 2 + 4 \cdot \frac{2}{3} = 0$$

$$\therefore M_D = 37.33 \text{ kN.m} \quad (\text{bending moment})$$

2- cross-section of (E):

Segment CE as F.B.D



$$+\uparrow \sum F_x = 0$$

$$N_E + Cy \cdot \sin 60^\circ = 0$$

$$\therefore [N_E = -13.86 \text{ kN}] \text{ (axial compressive force)}$$

$$+\uparrow \sum F_y = 0$$

$$V_E - Cy \cdot \cos 60^\circ = 0$$

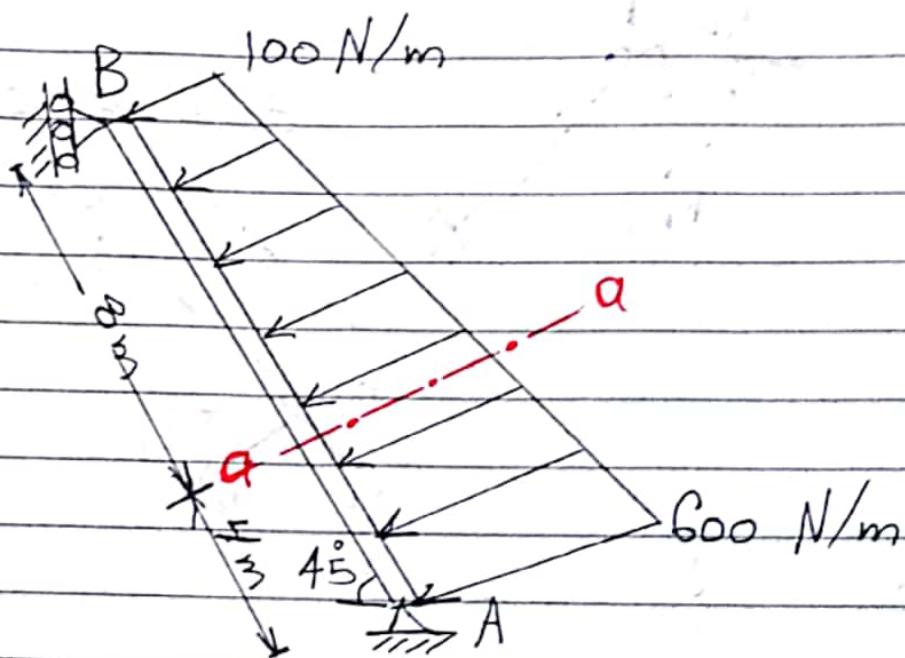
$$\therefore [V_E = 8 \text{ kN}] \text{ (shear force)}$$

$$+\odot \sum M_E = 0$$

$$M_E - (Cy \cos 60^\circ) \times 3 = 0$$

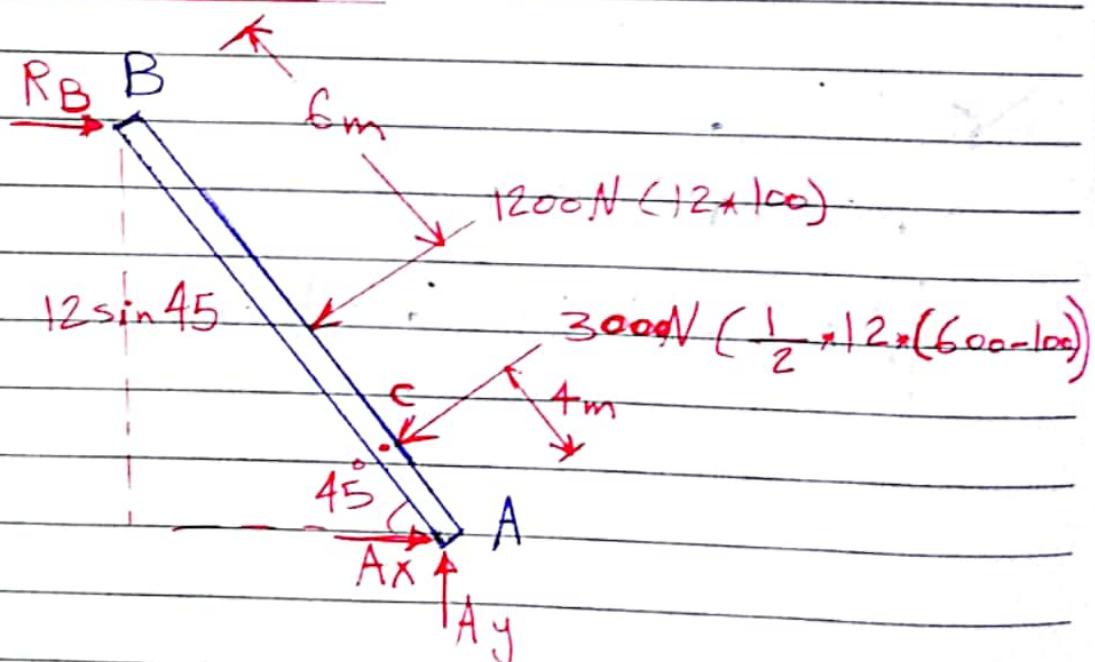
$$\therefore [M_E = 24 \text{ kN.m}] \text{ (Bending moment).}$$

Ex 2 : Determine the internal forces acting on section (a-a) off point (c), for the beam shown below.



Sol

Beam AB as F.B.D

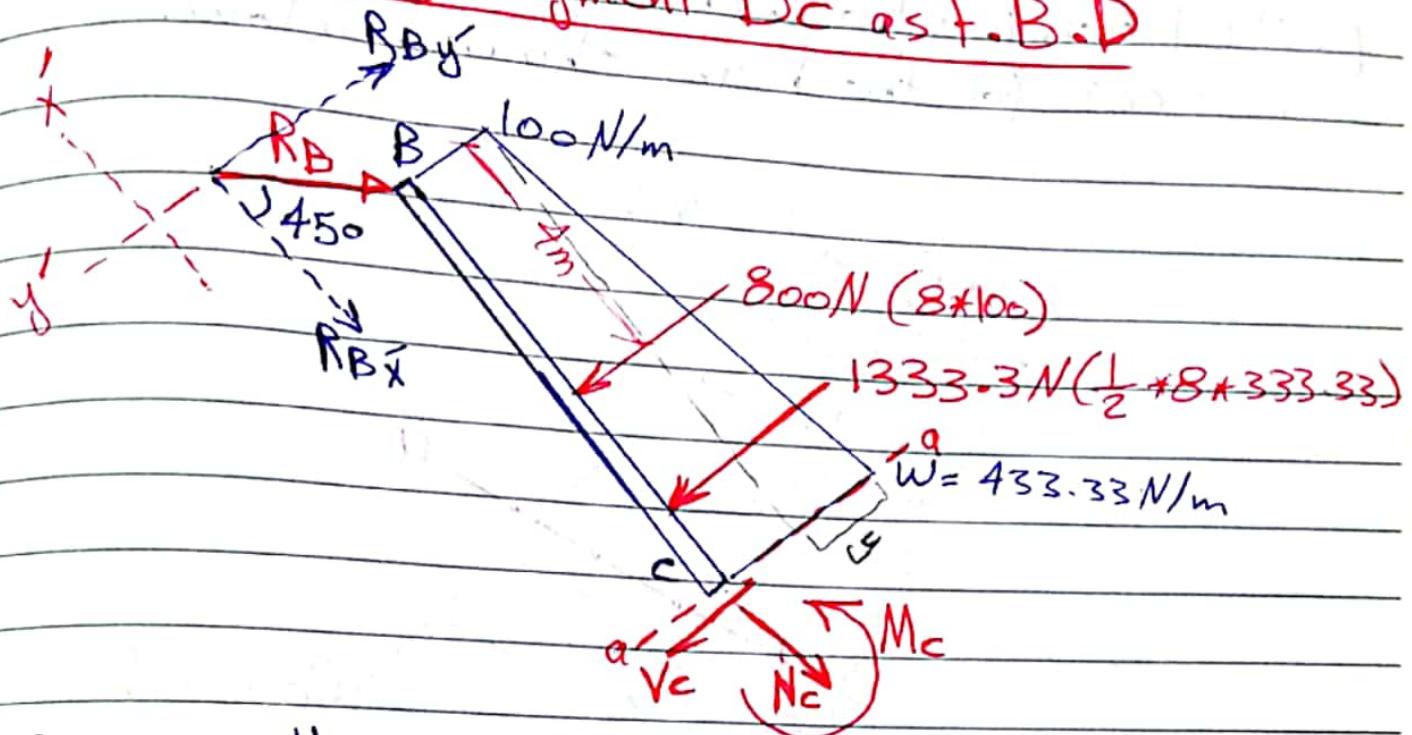


$$\text{At } \Sigma M_A = 0$$

$$R_B(12 \sin 45^\circ) - 1200 \times 6 - 3000 \times 4 = 0$$

$$\therefore R_B = 2263.10 \text{ N}$$

For Sec (a-a) Segment BC as F.B.D



$$\frac{500}{12} = \frac{y}{8} \Rightarrow y = 333.33 \text{ N/m}$$

$$w = y + 100 \Rightarrow w = 433.33 \text{ N/m}$$

$$+\sum F_x = 0$$

$$N_C + R_B \cos 45^\circ = 0$$

$$N_C = -1600 \text{ N} \quad (\text{axial compressive force})$$

$$+\nearrow \sum F_y = 0$$

$$R_B \cdot \sin 45^\circ - V_c - 800 - 1333 \cdot 3 = 0$$

$$V_c = -533.30 \text{ N}$$

$$+\sum M_c = 0$$

$$\frac{1}{3} * 8$$

$$M_c + 1333 \cdot 3 \left(8 - \left(\frac{2}{3} * 8 \right) \right) + 800 * 4 - (R_B * 8 \cdot \sin 45) = 0$$

$$M_c + 1333 \cdot 3 (8 - 5.333) + 800 * 4 - (2263 \cdot 1 \cdot 8 \cdot \sin 45) = 0$$

$$M_c = 6044.20 \text{ N} \cdot \text{m}$$