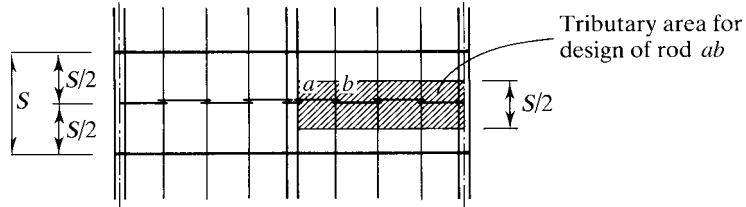


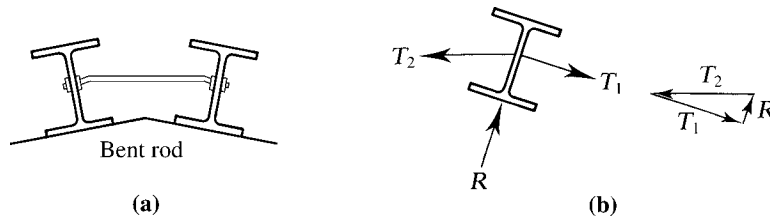
FIGURE 3.30



The extra amount of material in question is insignificant, and the use of the same size for each segment eliminates the possibility of a mix-up during construction.

A possible treatment at the peak or ridge is shown in Figure 3.31a. The tie rod between ridge purlins must resist the load from all of the sag rods on either side. The tensile force in this horizontal member has as one of its components the force in the upper sag-rod segment. A free-body diagram of one ridge purlin illustrates this effect, as shown in Figure 3.31b.

FIGURE 3.31



EXAMPLE 3.15

Fink trusses spaced at 20 feet on centers support W6 × 12 purlins, as shown in Figure 3.32a. The purlins are supported at their midpoints by sag rods. Use A36 steel and design the sag rods and the tie rod at the ridge for the following service loads.

| | |
|----------------|---|
| Metal deck: | 2 psf |
| Built-up roof: | 5 psf |
| Snow: | 18 psf of horizontal projection of the roof surface |
| Purlin weight: | 12 pounds per foot (lb/ft) of length |

SOLUTION

Calculate loads.

Tributary width for each sag rod = $20/2 = 10$ ft

Tributary area for deck and built-up roof = $10(46.6) = 466$ ft²

Dead load (deck and roof) = $(2 + 5)(466) = 3262$ lb

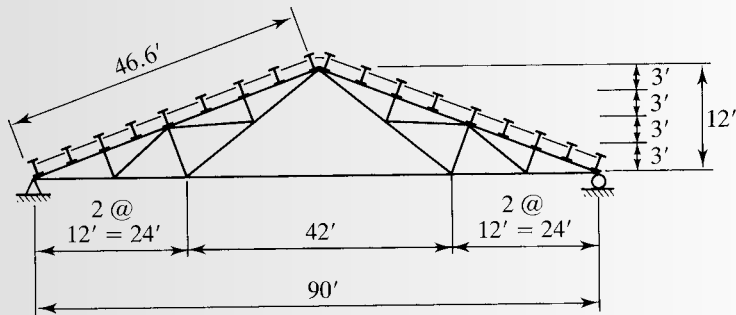
Total purlin weight = $12(10)(9) = 1080$ lb

Total dead load = $3262 + 1080 = 4342$ lb

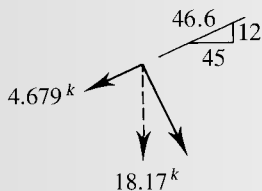
Tributary area for snow load = $10(45) = 450$ ft²

Total snow load = $18(450) = 8100$ lb

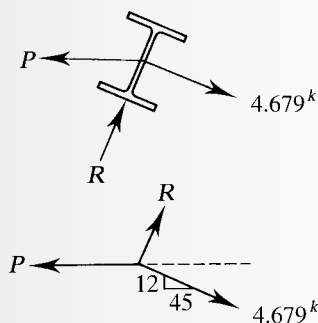
FIGURE 3.32



(a)



(b)



(c)

LRFD SOLUTION

Check load combinations.

Combination 2: $1.2D + 0.5S = 1.2(4342) + 0.5(8100) = 9260 \text{ lb}$

Combination 3: $1.2D + 1.6S = 1.2(4342) + 1.6(8100) = 18,170 \text{ lb}$

Combination 3 controls. (By inspection, the remaining combinations will not govern.)

For the component parallel to the roof (Figure 3.32b),

$$T = (18.17) \frac{12}{46.6} = 4.679 \text{ kips}$$

$$\text{Required } A_b = \frac{T}{\phi_t (0.75F_u)} = \frac{4.679}{0.75(0.75)(58)} = 0.1434 \text{ in.}^2$$

ANSWER

Use a $\frac{5}{8}$ -inch-diameter threaded rod ($A_b = 0.3068 \text{ in.}^2$).

Tie rod at the ridge (Figure 3.32c):

$$P = (4.679) \frac{46.6}{45} = 4.845 \text{ kips}$$

$$\text{Required } A_b = \frac{4.845}{0.75(0.75)(58)} = 0.1485 \text{ in.}^2$$