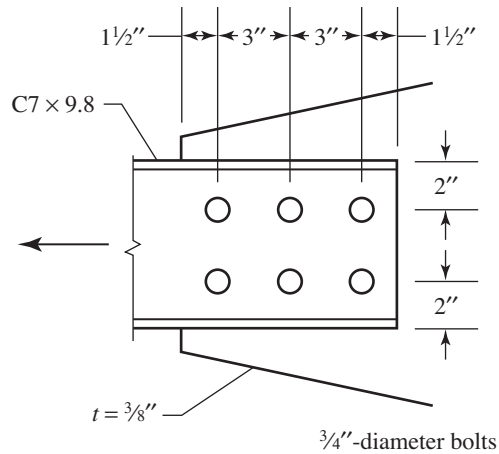
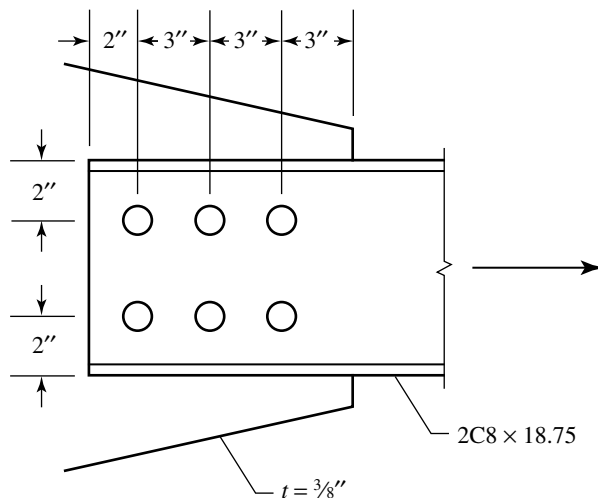


- 3.5-5** A $C7 \times 9.8$ tension member is connected to a $\frac{3}{8}$ -in.-thick gusset plate as shown in Figure P3.5-5. Both the member and the gusset plate are A36 steel.
- Compute the available block shear strength of the tension member for both LRFD and ASD.
 - Compute the available block shear strength of the gusset plate for both LRFD and ASD.

**FIGURE P3.5-5**

- 3.5-6** A double-channel shape, $2C8 \times 18.75$, is used as a tension member. The channels are bolted to a $\frac{3}{8}$ -inch gusset plate with $\frac{7}{8}$ -inch diameter bolts. The tension member is A572 Grade 50 steel and the gusset plate is A36. If LRFD is used, how much factored tensile load can be applied? Consider *all* limit states.

**FIGURE P3.5-6**

Design of Tension Members

- 3.6-1** Select a single-angle tension member of A36 steel to resist the following service loads: dead load = 50 kips, live load = 100 kips, and wind load = 45 kips. The member will be connected through one leg with 1-inch diameter bolts in two lines. There will be four bolts in each line. The member length is 20 feet.
- Use LRFD.
 - Use ASD.

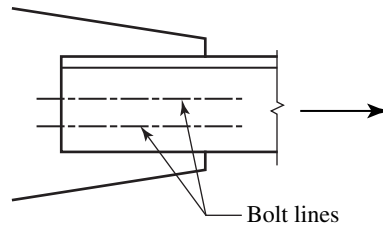


FIGURE P3.6-1

- 3.6-2** Use A36 steel and select a double-angle tension member to resist a service dead load of 20 kips and a service live load of 60 kips. Assume that the member will be connected to a $\frac{3}{8}$ -inch-thick gusset plate with a single line of five $\frac{7}{8}$ -inch diameter bolts. The member is 15 feet long.
- Use LRFD.
 - Use ASD.

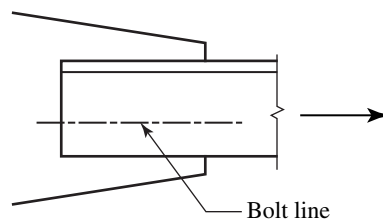


FIGURE P3.6-2

- 3.6-3** Select an ST shape to be used as a 20-ft-long tension member to resist the following service loads: dead load = 38 kips, live load = 115 kips, and snow load = 75 kips. The connection is through the flange with three $\frac{3}{4}$ -inch diameter bolts in each line. Use A572 Grade 50 steel.
- Use LRFD.
 - Use ASD.