

Limitations on Reinforcement Ratio for Doubly Reinforced beams

Lower limit on ρ (ACI 10.5)
same as for single reinforce beams.

$$\rho_{\min} = \frac{3\sqrt{f'_c}}{f_y} \geq \frac{200}{f_y}$$

Example: Doubly Reinforced Section

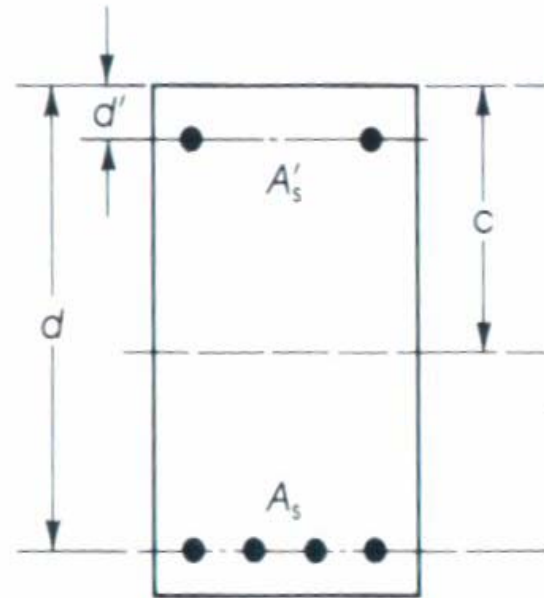
Given:

$$f'_c = 4000 \text{ psi } f_y = 60 \text{ ksi}$$

$$A'_s = 2 \text{ #5 } A_s = 4 \text{ #7}$$

$$d' = 2.5 \text{ in. } d = 15.5 \text{ in}$$

$$h = 18 \text{ in. } b = 12 \text{ in.}$$



Calculate M_n for the section for the given compression steel.

Example: Doubly Reinforced Section

Compute the reinforcement coefficients, the area of the bars #7 (0.6 in²) and #5 (0.31 in²)

$$A_s = 4(0.6 \text{ in}^2) = 2.4 \text{ in}^2$$

$$A'_s = 2(0.31 \text{ in}^2) = 0.62 \text{ in}^2$$

$$\rho = \frac{A_s}{bd} = \frac{2.4 \text{ in}^2}{(12 \text{ in.})(15.5 \text{ in.})} = 0.0129$$

$$\rho' = \frac{A'_s}{bd} = \frac{0.62 \text{ in}^2}{(12 \text{ in.})(15.5 \text{ in.})} = 0.0033$$

Example: Doubly Reinforced Section

Compute the effective reinforcement ratio and minimum ρ

$$\rho_{eff} = \rho - \rho' = 0.0129 - 0.0033 = 0.00957$$

$$\rho = \frac{200}{f_y} = \frac{200}{60000} = 0.00333 \text{ or } \frac{3\sqrt{f_c}}{f_y} = \frac{3\sqrt{4000}}{60000} = 0.00316$$

$$\rho \geq \rho_{min} \Rightarrow 0.0129 \geq 0.00333 \text{ OK!}$$

Example: Doubly Reinforced Section

Compute the effective reinforcement ratio and minimum ρ

$$\begin{aligned}(\rho - \rho') &\geq \left(\frac{\beta_1 (0.85 f'_c) d'}{d f_y} \right) \left(\frac{87000}{87000 - f_y} \right) \\ &\geq \left(\frac{0.85 (0.85 (4 \text{ ksi})) (2.5 \text{ in.})}{60 \text{ ksi} (15.5 \text{ in.})} \right) \left(\frac{87}{87 - 60} \right) = 0.0398\end{aligned}$$

$0.00957 \not\geq 0.0398$ Compression steel has not yielded.

Example: Doubly Reinforced Section

Compute the effective reinforcement ratio and minimum ρ

$$\begin{aligned} f'_s &= 29 \times 10^6 \left(1 - \frac{\beta_1 (0.85 f'_c) d'}{(\rho - \rho') d f_y} \right) 0.003 \\ &= 87 \left(1 - \frac{0.85 (0.85 (4 \text{ ksi})) (2.5 \text{ in.})}{(0.00957) (15.5 \text{ in.}) (60 \text{ ksi})} \right) = 16.37 \text{ ksi} \end{aligned}$$

Use an iterative technique to find f'_s

Example: Doubly Reinforced Section

Compute the iterative values (1)

$$c = \frac{A_s f_y - A'_s f'_s}{0.85 f_c \beta_1 b} = \frac{(2.4 \text{ in}^2)(60 \text{ ksi}) - (0.62 \text{ in}^2)(16.37 \text{ ksi})}{0.85(4 \text{ ksi})(0.85)(12 \text{ in.})}$$

$$c = 3.86 \text{ in.}$$

$$\begin{aligned} f'_s &= E_s \varepsilon'_s = 29 \times 10^6 \left(1 - \frac{d'}{c} \right) 0.003 = 87 \left(1 - \frac{(2.5 \text{ in.})}{(3.86 \text{ in.})} \right) \\ &= 30.65 \text{ ksi} \end{aligned}$$

Use an iterative technique to find f'_s

Example: Doubly Reinforced Section

Compute the iterative values (2)

$$c = \frac{A_s f_y - A'_s f'_s}{0.85 f_c \beta_1 b} = \frac{(2.4 \text{ in}^2)(60 \text{ ksi}) - (0.62 \text{ in}^2)(30.65 \text{ ksi})}{0.85(4 \text{ ksi})(0.85)(12 \text{ in.})}$$

$$c = 3.604 \text{ in.}$$

$$\begin{aligned} f'_s &= E_s \varepsilon'_s = 29 \times 10^6 \left(1 - \frac{d'}{c} \right) 0.003 = 87 \left(1 - \frac{(2.5 \text{ in.})}{(3.604 \text{ in.})} \right) \\ &= 26.66 \text{ ksi} \end{aligned}$$

Use an iterative technique to find f'_s

Example: Doubly Reinforced Section

Compute the iterative values (2)

$$c = \frac{A_s f_y - A'_s f'_s}{0.85 f_c \beta_1 b} = \frac{(2.4 \text{ in}^2)(60 \text{ ksi}) - (0.62 \text{ in}^2)(30.66 \text{ ksi})}{0.85(4 \text{ ksi})(0.85)(12 \text{ in.})}$$

$$c = 3.676 \text{ in.}$$

$$\begin{aligned} f'_s &= E_s \varepsilon'_s = 29 \times 10^6 \left(1 - \frac{d'}{c} \right) 0.003 = 87 \left(1 - \frac{(2.5 \text{ in.})}{(3.676 \text{ in.})} \right) \\ &= 27.82 \text{ ksi} \end{aligned}$$

Use an iterative technique to find f'_s

Example: Doubly Reinforced Section

Compute the iterative values (etc)

$$c = \frac{A_s f_y - A'_s f'_s}{0.85 f_c \beta_1 b} = \frac{(2.4 \text{ in}^2)(60 \text{ ksi}) - (0.62 \text{ in}^2)(27.49 \text{ ksi})}{0.85(4 \text{ ksi})(0.85)(12 \text{ in.})}$$

$$c = 3.66 \text{ in.}$$

$$\begin{aligned} f'_s &= E_s \varepsilon'_s = 29 \times 10^6 \left(1 - \frac{d'}{c} \right) 0.003 = 87 \left(1 - \frac{(2.5 \text{ in.})}{(3.66 \text{ in.})} \right) \\ &= 27.57 \text{ ksi} \end{aligned}$$

Use $f'_s = 27.56 \text{ ksi}$ $c = 3.66 \text{ in}$

Example: Doubly Reinforced Section

Compute the moment capacity of the beam

$$\begin{aligned} M_n &= \left(A_s f_y - A'_s f'_s \right) \left(d - \frac{a}{2} \right) + A'_s f'_s (d - d') \\ &= \left((2.4 \text{ in}^2)(60 \text{ ksi}) - (0.62 \text{ in}^2)(27.56 \text{ ksi}) \right) \left(15.5 \text{ in.} - \frac{0.85(3.66 \text{ in.})}{2} \right) \\ &\quad + (0.62 \text{ in}^2)(27.56 \text{ ksi})(15.5 \text{ in.} - 2.5 \text{ in.}) \\ &= 1991.9 \text{ k-in.} \Rightarrow 166 \text{ k-ft} \end{aligned}$$