

SubjectStrength of MaterialsStageSecond stageLecturerDr. Mujtaba A. FlayyihCodeUOMU013043

2<sup>nd</sup> semester (2024-2025)

## Lecture No. 3 Deflection of the beam

## Introduction

When we consider designing beams based on rigidity consideration the deflection of the beam must be known at specific or critical location. Several methods are available for determining beam deflection. Although based on the same principles, they differ in technique and in their immediate objective.

Methods of Determining Beam Deflections

Numerous methods are available for the determination of beam deflections. These methods include:

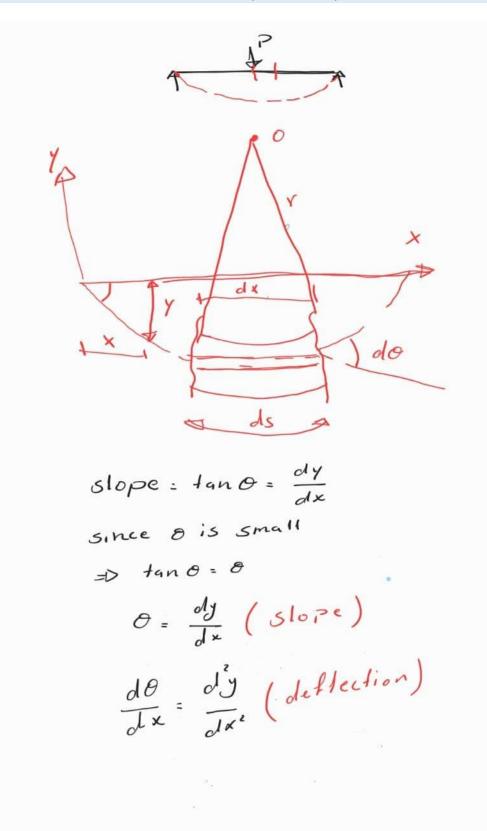
- Double-integration method
- Area-moment method
- Strain-energy method (Castigliano's Theorem)
- Conjugate-beam method
- Method of superposition

## **Double-integration method**

The double integration method is a powerful tool in solving deflection and slope of a beam at any point because we will be able to get the equation of the elastic curve. *The double integration method can be used if the moment* (M) *has a single expression for the whole beam.* 



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The differential length of the  
strip ds can be expressed in  
terms of radius of curvature  

$$dS = r \star d\theta$$

$$= \frac{1}{r} = \frac{d\theta}{ds} \approx \frac{d\theta}{dx} \qquad (1)$$
because the deflection of the  
beam is very small  
(ds  $\approx$  dx)  
From the basic law of bending  

$$\frac{M}{I} = \frac{\sigma_b}{Y} = \frac{E}{r}$$

$$= \frac{M}{I} = \frac{E}{r}$$

$$= \frac{M}{I} \approx \frac{E}{r}$$

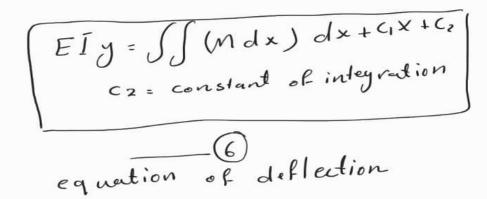


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since 
$$\frac{d\theta}{dx} = \frac{d^2y}{dx^2}$$
  
 $\Rightarrow \frac{d^2y}{dx^2} = \frac{M}{EI}$   
 $\Rightarrow EI \frac{d^2y}{dx^2} = M - 4$   
equation of bending  
by differiation of eq. (4)  
we get the slope of the  
beam  
 $\Rightarrow EI \frac{dy}{dx} = \int M dx + C$   
 $c = constant of integration$   
equation of slope (5)  
another differiention of the  
equation (5), we get the  
deflection of beam



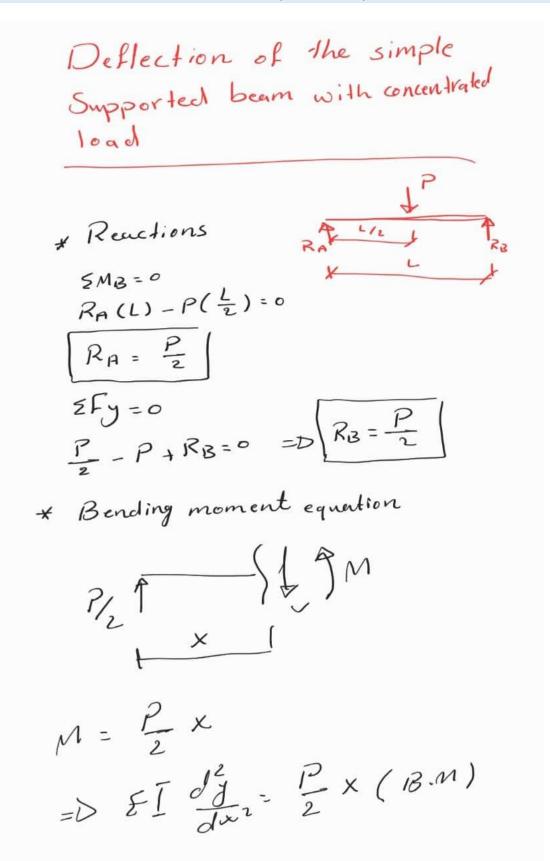
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$$EI \frac{dY}{dx} = \frac{P}{4} x^{2} + C_{1}(\delta | ope)$$

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$$EI \frac{dY}{dx} = \frac{P}{12} x^{3} + C_{1}x + C_{2}$$

$$(deflection)$$

$$Boundary condition$$

$$ot x = 0 \Rightarrow Y = 0$$

$$= D C_{2} = 0$$

$$ot x = \frac{L}{2} = D \frac{dY}{dx} = 0$$

$$o = \frac{P}{4} (\frac{L}{2})^{2} + C_{1}$$

$$= D \frac{C_{1}}{C_{1}} = \frac{-PL^{2}}{16}$$

$$= D The \sigma | ope equation$$

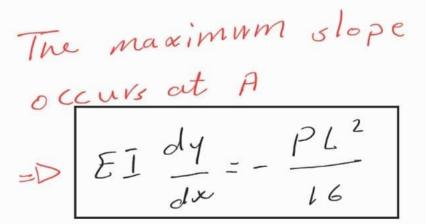
$$\left(\frac{FI}{dx} = \frac{P}{4} x^{2} - \frac{PL^{2}}{16}\right)$$



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The deflection equation  $y = \frac{P x^3}{12} - \frac{P L^2}{16}(x)$ 



The maximum deflection occurs at centre of the beam  $(\chi = \frac{L}{2})$ 

$$= D$$

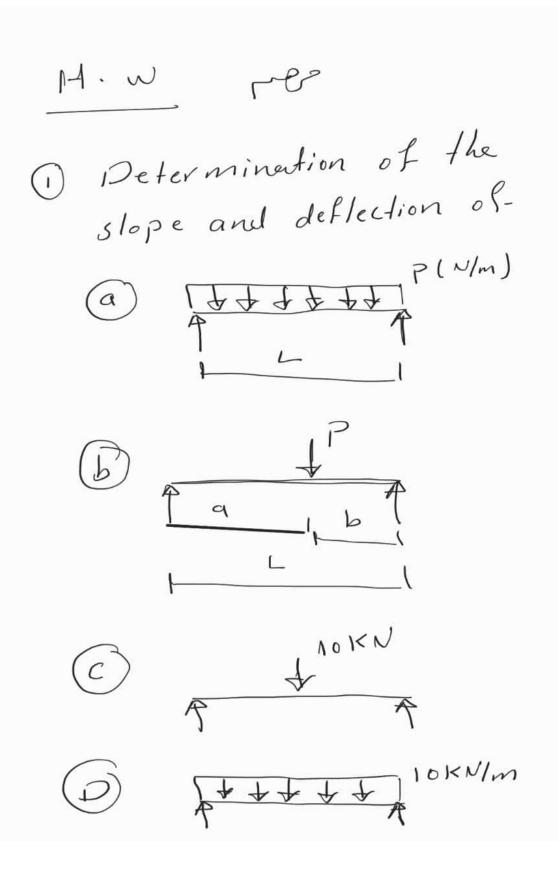
$$EI y = \frac{P}{12} \left(\frac{L}{2}\right)^3 - \frac{PL^2}{16} \left(\frac{L}{2}\right)$$

$$= \frac{PL^3}{16} \left(\frac{L}{2}\right)$$



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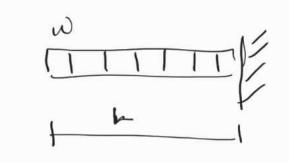
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Deflection & slope of-Cantilever beam with disturbuted load





$$M = -\frac{\omega x^2}{z}$$

$$W = -\frac{\omega x^2}{z}$$

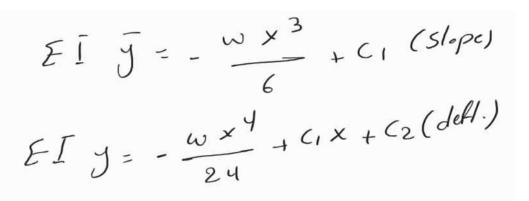
$$= \mathcal{D} \mathcal{E} \tilde{I} \tilde{Y} = - \frac{\mathcal{W} X}{2} (\mathcal{B} \cdot \mathcal{M})$$

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 $B \cdot C$   $at x = L \rightarrow \bar{y} = 0$   $C = -\frac{\omega L^3}{6} + c_1$   $C_1 = -\frac{\omega L^3}{6}$ 

at 
$$w = L = 2 Y = 0$$
  

$$o = -\frac{wL^{4}}{24} + \frac{wL^{3}}{6}L + C_{p}$$

$$= 24 - \frac{wL^{4}}{6}$$



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$$\hat{e} = \frac{-\omega x^3}{6} + \frac{\omega L^3}{6}$$

$$EIy = -\frac{wx^{4}}{24} + \frac{wL^{3}}{6}x - \frac{wL^{4}}{8}$$

e



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